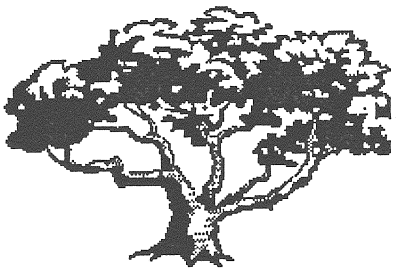
Joshua Heights

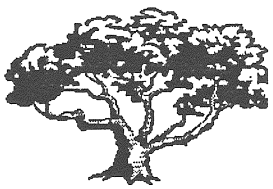
Bridgewater, Connecticut



King's Mark Environmental Review Team Report

King's Mark Resource Conservation and Development Area, Inc.

Joshua Heights
Bridgewater, Connecticut



Environmental Review Team Report

Prepared by the
King's Mark Environmental Review Team
of the
King's Mark
Resource Conservation and Development Area, Inc.

for the
Inland Wetlands Commission
Bridgewater, Connecticut

May 1997

CT Environmental Review Teams
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Acknowledgments

This report is an outgrowth of a request from the Bridgewater Inland Wetlands Commission to the Litchfield County Soil and Water Conservation District (SWCD). The SWCD referred this request to the King's mark Resource Conservation and Development Area (RC&D) Executive Council for their consideration and approval. The request was approved and the measure reviewed by the King's Mark Environmental Review Team (ERT).

The King's Mark Environmental Review Team Coordinator, Elaine Sych, would like to thank and gratefully acknowledge the following Team members whose professionalism and expertise were invaluable to the completion of this report.

The field review took place on Thursday, March 27, 1997.

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I would also like to thank Alex McNaughton, chairman of the Bridgewater Inland Wetlands Commission and John Carr, the applicant and engineer for their cooperation and assistance during this environmental review.

Prior to the review day, each Team member received a summary of the proposed project with location and soils maps. During the field review Team members were given additional plans and information. Following the review, reports from each Team member were submitted to the ERT coordinator for compilation and editing into this final report.

This report represents the Team's findings. It is not meant to compete with private consultants by providing site plans or detailed solutions to development problems. The Team does not recommend what final action should be taken on a proposed project - all final decisions rest with the Town. This report identifies the existing resource base and evaluates its significance to the proposed development, and also suggests considerations that should be of concern to the Town. The results of this Team action are oriented toward the development of better environmental quality and the long term economics of land use.

The King's Mark RC&D Executive Council hopes you will find this report of value and assistance in making your decisions on this proposed housing project.

If you require additional information please contact:

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Introduction

Introduction

The Bridgewater Inland Wetlands Commission has requested assistance from the King's Mark Environmental Review Team in conducting an environmental review of the proposed Joshua Heights residential housing project.

The 24 acre site is located on the east side of Route 133, approximately 1/4 mile south of Route 67 in Bridgewater. The project consists of an affordable housing development to be constructed on 14 acres on three adjoining lots. Lot 1 is +4 acres in size and will consist of four (4) quadriplexes, Lot 2 is +3 and will contain five (5) duplexes, and Lot 3 which is +6 in size will have nine (9) single family homes. All will be served by individual on-site sewage disposal systems and wells.

Objectives of the ERT Study

The Inland Wetlands Commission is concerned with the impact of this development on wetlands with regard to the proposed 18 sewage disposal systems and the impact of stormwater from roads and other impervious surfaces. Other concerns include water supply and water quality, and any other potential negative impacts to wetlands or Clapboard Oak Brook.

The water supply issue is also being studied and analyzed by the CT Department of Public Health Water supply Section (contact Michael Hage, 860-509-7333) and the CT Department of Public Utilities Control separate from this Team review.

The ERT Process

Through the efforts of the Inland Wetlands Commission this environmental review and report was prepared for the Town of Bridgewater.

This report provides an information base and a series of recommendations and guidelines which cover the topics requested by the Town. Team members were able to review maps and supporting documentation provided by the applicant.

The review process consisted of four phases:

1. Inventory of the site's natural resources;
2. Assessment of these resources;
3. Identification of resource areas and review of plans; and
4. Presentation of education, management and land use guidelines.

The data collection phase involved both literature and field research. The field review was conducted on March 27, 1997. The emphasis of the field review was on the exchange of ideas, concerns and recommendations. Being on site allowed Team members to verify information and to identify other resources.

Once Team members had assimilated an adequate data base, they were able to analyze and interpret their findings. Individual Team members then prepared and submitted their reports to the ERT coordinator for compilation into this final ERT report.

Figure 1

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Location and Topographic Map

Scale 1" = 2000'

—— Approximate Site

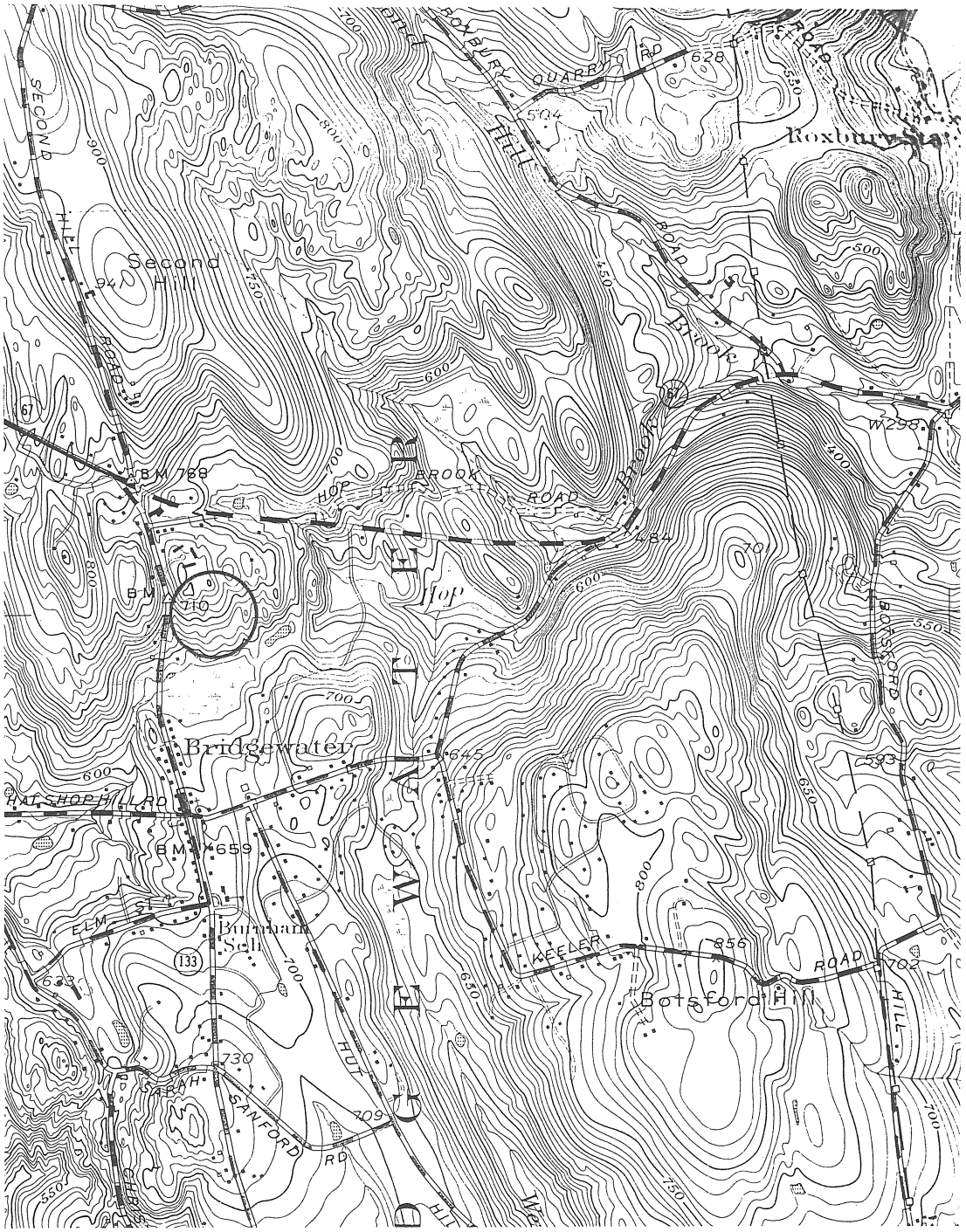

Figure 2

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N

Soils Map

Scale 1" = 1320'

Topography, Geology and Hydrogeology

Topography and Surficial Geology

The 24 acre site of the proposed Joshua Heights development lies on a south facing bedrock slope. A series of south-southeast trending bare bedrock ridges, separated by narrow, 100-150 foot wide valleys characterize the local topography. The valleys and irregular depressions along the ridges are filled by up to 15 feet of loose poorly sorted sandy till. Large angular boulders, up to several feet in diameter, are strewn across the surface. Small, circular pothole-like water-worn depressions cut vertically into the bedrock along some ridges suggest that rapidly flowing glacial meltwaters stripped the hillside of its original glacially deposited sediments. Indeed, the distinct topography of the site probably reflects significant erosion and sculpting by subglacial meltwaters. Other south facing slopes in the area are less ravined and topographically much smoother. Presumably, as the continental ice sheet melted 14,000 years ago, a temporary subglacial cavern which opened on the shielded southern side of the Second Hill drumlin was filled with rapidly flowing meltwaters which incised the closely spaced ravines into the bedrock and redeposited the compact basal glacial till previously covering the site. The large boulders melted out of the of the melting ice long after the meltwaters from up glacier had found other routes around decaying blocks of stagnant ice which clogged the normal drainage system.

Bedrock Geology

The Joshua Heights site is underlain by a homogeneous well foliated, white colored granite gneiss made up of roughly equal amounts of the minerals; microcline ($KAlSi_3O_8$), albite ($NaAlSi_3O_8$), quartz (SiO_2), together with smaller amounts of the micas; muscovite ($KAl_3Si_3O_{10}(OH)_2$) and biotite ($K(Mg,Fe)_3AlSi_3O_{10}(OH)_2$). The granite is part of a 3 x 1.5 mile Mine Hill granite gneiss "stock" which is intrusive into the surrounding metamorphosed quartzite and mica schists of the Hartland Group (see Gates 1959). Although the Mine Hill Granite has been quarried in Roxbury, and some stone appears to have been quarried from bedrock ridges in the southeastern section

of this property, the granite bedrock over much of the area is much too schistose to make an attractive dimension stone. The southwestern edge of the Mine Hill stock runs just west of North Main Street at the western boundary of the Joshua Heights property (see geologic map, Figure 3). Along its margin both the granite and surrounding country rocks have been intensely deformed and a strong foliation paralleling the contact has developed.

Hydrogeology

Water wells drilled into the bedrock in the Bridgewater area produce all their water from the bedrock fractures. Yield is a function of the number and interconnectedness of fractures intersected by a well. As the density of fractures in granite is typically an order of magnitude less than that of normal metamorphic bedrock the feasibility of providing sufficient water for the Joshua Heights development, which is underlain entirely by granite, might be a cause for some concern. Two distinct joint sets were seen in the exposed bedrock on the site. One set of joints is vertical and strikes NE-SW. The other parallels the intense foliation, dips 40 degrees to the west and strikes roughly north-south. Joints of the vertical set are spaced meters apart and would be unlikely to be intersected by a vertical well. On the other hand, the north-south set is spaced more closely and with a dip of 40 degrees are guaranteed to be intersected by any well. As the intensity of the foliation parallel joint set increases towards the granite contact along North Main Street it would seem a reasonable bet that wells drilled on the property would provide an adequate supply of water. Indeed, a number nearby wells drilled into the same granite some distance from its contact, have proven to be productive. On air photos and the quadrangle map a set of well defined NNW topographic lineaments can be recognized. Although no joints paralleling to these features were seen in the bedrock exposures examined in the field, these are probably zones of intense fracturing. One such lineament follows the deep ravine along Route 133 and another is mapped out by a number of linear stream valleys just east of the property. Wells drilled along the course of these fracture zones could prove tremendously productive. The downside of that strategy however, is that the water quality from such wells may be less than desirable. The highly permeable fracture zones may have been the sites of hydrothermal

mineral deposition (for example the Roxbury iron mine veins follow that trend) or they may tap waters originating in the sulfide bearing rocks of the Hartland formation. In either case the waters may be unacceptably iron rich and/or acidic.

Recommendations

The thickness of overburden is highly variable over the site ranging from zero to perhaps in excess of 15 feet along the subglacially eroded small valleys. The loose sandy till filling the valleys may contain highly permeable lenses. Groundwater recharge is most likely along westward dipping fractures. These facts should be carefully considered in the siting of proposed septic systems and water wells.

References

E. Malde, 1967, Surficial Geological Map of the Roxbury Quadrangle, Litchfield and New Haven, Connecticut. USGS GQ-611.

M. Gates, 1959, Geologic map of the Roxbury Quadrangle, Connecticut; Bedrock Geology. USGS GQ-121.

Terminology

Drumlin
A streamlined hill consisting of glacially deposited sediment and elongated parallel with the direction of ice flow.

Foliation
The planar texture of mineral grains, principally micas, produced by metamorphism and deformation.

Schistosity
The parallel arrangement of coarse grains of the sheet-structure minerals, like mica and chlorite, formed during metamorphism under conditions of differential stress.

Hydrothermal mineral deposit
Any local concentration of minerals formed by deposition from a hot circulation brine solution.

Stock
A small, irregular body of intrusive igneous rock, smaller than a batholith, that cuts across the layering of the intruded rock.

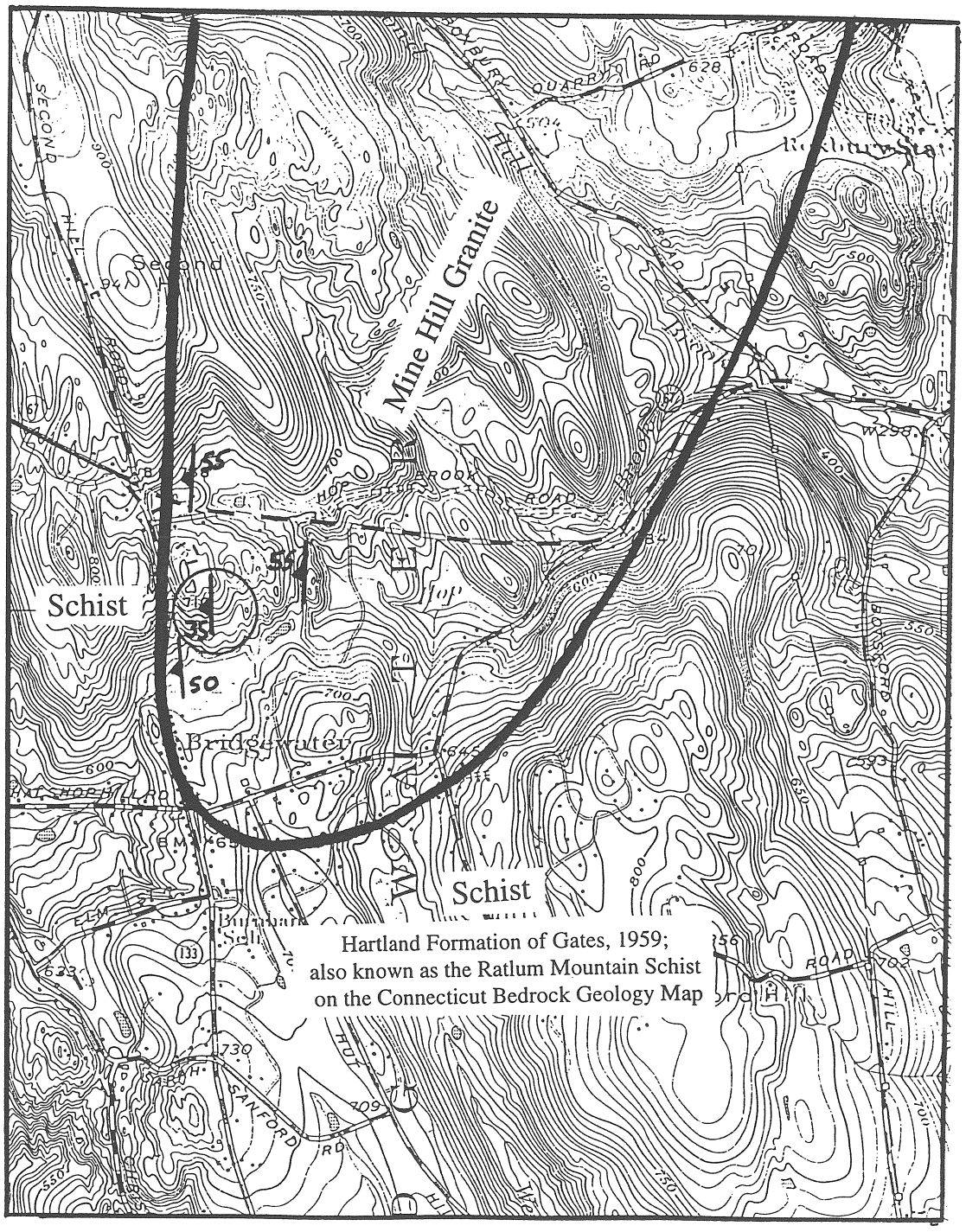
Figure 3

Bedrock Geology

(Geology of a portion of the Roxbury Quadrangle (after Gates, 1959))

Scale 1" = 2000'

——— Approximate Site



Wetland Resources

Included in this section are observations of the wetland resources, the impacts that the proposed activities may have on those resources and recommendations for future development of this parcel given these possible impacts.

Existing Conditions

On site wetland resources include a portion of a large, continuos, densely vegetated scrub/shrub headwater wetland entirely described as a "peat and muck" soil map unit within the Soil Survey of Litchfield County (1970). This soil map unit is comprised of deep, unconsolidated, decomposed plant materials that were deposited in what once was an open body of water. An excavated trench running along the northern edge of this wetland was detected from aerial photographs examined. This wetland drains under Rt. 133, generally in a southwesterly direction to form Clapboard Oak Brook, which flows for approximately 2.5 miles to join the Housatonic River. Upland sections of this parcel are comprised of well-drained and extremely well-drained soils on an average slope of 14% (roughly calculated off of the USGS quadrangle map).

Wetland Functional Values

A prime function of this type of wetland is storm water storage and flood control. Peaty materials are well known for absorbing many times their own weight in water, retaining that water and slowly releasing it. This action tends to "buffer" the effects of storm flows, naturally slowing its passage down through its watershed.

This wetland should also be highly valued for its beneficial effect on the quality of the ground and surface water that enters it from upland sections of its watershed. These densely vegetated, wetlands provide a large amount of surface area, the most important factor when evaluating the ability of a wetland to remove, store and naturally redistribute excessive stormwater nutrients and other pollutants.

Impact of Proposed Activities on Watercourses and Wetlands

There are no direct wetland impacts proposed. Indirect impacts to on and off-site wetlands as a result of the proposed development could possibly include decreasing wetland water quality resulting from excessive pollutants as part of permanent stormwater discharges, excessive effluent from poorly designed septic systems, and temporary sedimentation during the construction process. Each of these possible indirect impacts are more specifically treated under other sections of this ERT report, however, at the risk of being redundant the following comments and recommendations are offered.

Comments and Recommendations

As proposed, stormwater is being collected from the primary access road as well as a portion of the interior private access roads and parking lots. It is not clear if the roof runoff is being collected in the stormwater management system. Stormwater pollutants to be expected from such surfaces include suspended solids, hydrocarbons, excessive pesticides and nutrients from overland flow off of the lawns, salts and heavy metals. Currently, Best Management Practices (BMP's) for the treatment of stormwater includes what is being called "Stone Tiered Steps" at the single stormwater outlet proposed. The outlet is 35 feet from the wetland boundary and the Stone Tiered Steps will be placed at the outlet and continue to the wetland boundary. There are no detailed drawings for this treatment devise are included on the plan.

The Team Wetland Specialist has not been previously aware of such a device, but it appears that this is more of a grade stabilization measure than one for water quality treatment. Its benefits may include erosion and sedimentation control and aeration, however, it most likely will not adequately remove the other pollutants described above. In a previously proposed plan, the applicant intended to use a "biofiltration trench" which is basically a dry grassed swale. This measure, used alone as described, has limited water quality improvement capabilities. In their most recent update of urban stormwater quality BMP's, the Center for Watershed

Protection included this measure on their list of BMP's that do not fully meet water quality renovation goals.

After considering the nature of this site and the high value of the wetland resource to be protected, the most appropriate and effective BMP for this location may be what is called an "infiltration trench" (refer to Figure 4 for further information). This system would utilize the well-draining characteristics of the natural soils here, while at the same time re-introduce a portion of the stormwater back into the ground to flow more naturally as groundwater into the adjacent wetlands. This infiltration trench would accept only the "first flush" of stormwater that contains a majority of the pollutants, typically that volume which equals the first one inch of impervious runoff. Prior to flowing into the trench, stormwater should be pre-treated by one of the "new generation" sedimentation chambers now out on the market. These new designs limit re-suspension of collected sediments and allow for "first flush" stormwater by-passes. Other criteria include a maximum 8% slope, flat bottom trenches and two feet separation to the water table. Use of this BMP may require the re-designing of a small portion of this site to accommodate the above stated guidelines.

To further prevent indirect impact to wetlands as a result of erosion and sedimentation during the construction period, the following items should be added to the site plan:

- A schedule of major construction activities (including erosion and sedimentation control measures), preferably in the form of a "Gant" type bar chart listing each activity in proper sequence and assigning to them start and stop dates,
- A locus map of the project at a scale of 1" = 2000' including project limits, north arrow, street names, major drainage ways and watershed limits,
- References for horizontal and vertical control, property lines, floodplain boundaries, inland wetland boundaries, SCEL lines, etc.,
- Location, size and variety of notable trees,
- Temporary erosion protection when time of year or weather prohibit establishment of permanent vegetative cover,

- Planned permanent vegetation including landscaping plan, seed mixtures, mulch types, fertilizer requirements and proposed planting dates,
- Planned temporary vegetation if disturbed areas are to remain for thirty (30) days or more,
- Maintenance requirements of temporary measures during construction period including the name and phone number of the person responsible for this maintenance.

The site plan does include a reference to Connecticut's Guidelines for Soil Erosion and Sediment Control to be used as a "controlling document", however, it is preferred to have any pertinent information transferred from the guidelines and adapted to the site plan itself for easy reference in the field.

In addition, the construction sequence mentioned above should be included in a more detailed phasing sequence for this project. It is not clear from the plan if lot construction is to begin only after the primary road is completed or if these activities are to occur simultaneously. If feasible, the lot construction itself should be phased, to limit the area of disturbed soil at any one time.

Finally, a pre-construction meeting between the applicant, his consultants and municipal staff members should occur to improve coordination between these parties as construction proceeds.

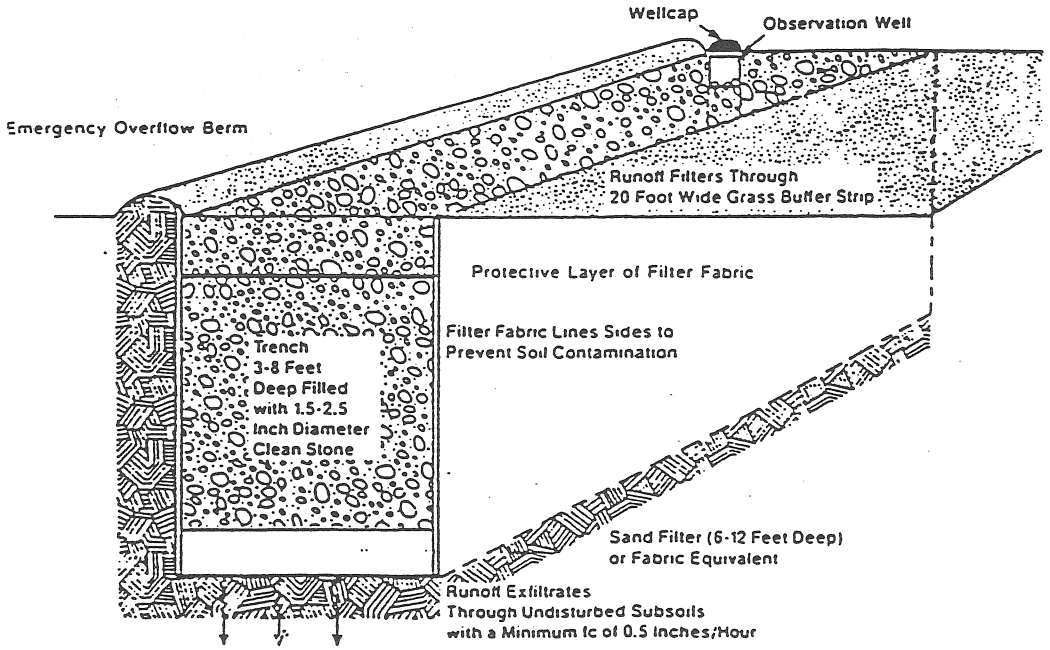
Figure 4

Definition

A conventional infiltration trench is a shallow, excavated trench that has been backfilled with stone to create an underground reservoir. Stormwater runoff diverted into the trench gradually exfiltrates from the bottom of the trench into the subsoil and eventually into the water table.

Enhanced infiltration trenches have extensive pretreatment systems to remove sediment and oil. They require on-site geotechnical investigations to determine appropriate design and location.

Schematic Design of a Conventional Infiltration Trench



Source: Schueler, 1987.

Stormwater Management

The 24 acre site slopes at a 10-15% grade to the south and east and is bordered by a large wetland in that direction. Approximately 14 acres of the site will be disturbed for the residential development. The wetland seems to comprise the headwaters of Clapboard Oak Brook which flows into the Housatonic River approximately +2 miles west of the site.

The development is divided into three (3) lots - one with quadriplexes, one with duplexes and one with single family units. The site will be served by a proposed cul-de-sac road to extend approximately 750 feet east from Route 133. The lots will be served by a network of small cul-de-sac driveways extending from the main road. Lot 1 is the northernmost lot and consists of four (4) quadriplex units with their associated wells and septic systems. Drainage from this lot is not collected in a drainage system and flows overland to the southeast and into the adjacent wetlands. Lot 2 is the middle lot and consists of five (5) duplex units with their associated wells and septic systems. Drainage from this lot mostly flows overland to the southeast and into adjacent wetlands. Some of the drainage is collected in the driveway system and discharges to the outfall at the south end of the site. Lot 3 is the southernmost lot and consists of nine (9) single family units with associated wells and septic systems. Drainage south of the driveways and north of the driveways is collected in the drainage system through catchbasins in the driveways and flows to the trunk line which discharges at the south end of the site. All of the road drainage for the cul-de-sac and the wooded area north of the proposed entrance road is collected by the roadway drainage system and flows into the same trunk line discharging to the south.

Potential runoff pollutant, including sediment, in any overland flows should be properly mitigated by the vegetation over which the runoff flows before it reaches the wetland. Any negligible remaining pollutants should be mitigated within the wetland before they reach Clapboard Brook. Pollutants in the runoff collected in the stormdrain system will be discharged at the south outfall. At present, the plans show a stepped riprap outfall pad at this location. The pipe leading to this discharge is at a steep grade and this pad may not be adequate for energy dissipation. Any energy dissipater should be installed or calculations provided showing the adequacy of the proposed pad

to properly dissipate the energy of the outfall. If possible, the length of this pad should be extended and planted to allow some aeration and plant uptake of potential pollutants prior to discharge into the wetlands (Please also refer to Wetland Resources section for additional comments). In addition, a sediment and floatables control structure utilizing swirl concentration technology or equal should be installed prior to discharge. The best location for this structure would be at the last manhole in the stormdrain trunk line. A regular maintenance schedule should also be specified for the drainage system.

A registration for the General Permit for the Discharge of Stormwater and Dewatering Wastewaters from Construction Activities must be submitted at least 15 days prior to the start of construction. A Stormwater Pollution Control Plan must also be prepared and kept on site at that time. In particular, erosion and sediment control measures utilized must be appropriate for a steeply graded site. Temporary construction sedimentation basins must be employed with all disturbed areas draining into the basin. The plan currently does not show any sedimentation basins. Sedimentation basins shall have a capacity of at least 134 cubic yards per acre drained to them. Disturbed areas to be left for over 30 days will receive temporary seeding or heavy mulch. All disturbed areas must be seeded as soon as possible. No areas may be left bare by the end of the planting season. On a site as steep as this, care must be taken to properly stabilize seeded areas with mulch and/or geotextiles. Properly constructed and maintained, the site should have no measurable impact on the adjacent wetlands or downstream watercourses.

Sewage Disposal

A preliminary review of information submitted to ERT Team members has been conducted. Three types of residential developments are proposed on each of three residential parcels. Prior to being notified of this ERT review, approximately two hours was spent by the Team sanitary engineer with the developer and town sanitarian, Thomas O'Loskey, reviewing and discussing preliminary site development plans.

Soils on the 24 acre parcel would normally be considered suitable for standard residential development. What makes this proposal somewhat unique is that the overall density of development is significantly higher than usually seen with single family subdivision layouts. With that increased density comes concern for the impact of sewage disposal systems on groundwater quality, overall hydraulic capacity of the soils and protection of the 18 proposed wells shown on site development plans. These items as well as state agency jurisdiction for review will be handled independently in the following discussion:

- Jurisdiction For State Review/Approval - Normally, large projects such as this would fall under the direct supervision of the Department of Environmental Protection which must review and approve all discharges to the groundwater's of our State which exceed 5,000 gallons per day. The three proposed residential developments total 89 bedrooms with a capacity to discharge approximately 13,000 gallons per day. What is unique about this site is that the developer has indicated each of the proposed parcels will be owned by a separate corporation not associated with the other, therefore, each of the individual lots fall just under the 5,000 gallon per day jurisdictional line thereby necessitating review for the three individual lots by State and Local Health Departments only. Provisions in the Public Health Code do allow both State and Local Health Departments to ask for the exact same technical review normally required in larger DEP projects. The town has secured services of a consulting engineer to assist them with this review in making the

determination as to which specific areas of analysis should be requested of the developer. The State Department of Public Health supported the town in obtaining these services for a second opinion.

- Density of Development - On normally suitable 14 acre sites, a developer might expect to create 10 or 12 residential lots, each suitable for a three or four bedroom residence. Based upon those numbers, Joshua Heights represents a doubling or tripling of domestic sewage flows normally associated with development. The impact of this densely developed project has a potential for adversely affecting groundwater quality on and adjacent to the lots being developed. While this may adversely affect the water quality down gradient at the lower property line, the most critical aspect is protection of groundwater supplies relied upon for development of potable wells.

  Nitrogen generated in domestic sewage is only slightly reduced by standard onsite disposal systems and relies upon dilution by rain water in order to minimize the affect on ground and surface water. With limited watershed upgradient, nitrates may be a problem on one or two of these parcels. Information has been requested of the engineer to address these concerns and the town's consultant is also analyzing this information. Normally this analysis is not even considered on standard subdivision lots where a single home is proposed on lots of one acre or larger.

- Hydraulic Capacity - The Hollis soils identified over much of the site, with slopes averaging 7%, are generally well suited for construction of small subsurface sewage disposal systems. Bedrock was observed in several of the test holes on the site. The Connecticut Public Health Code relies upon application of the Department of Public Health's Minimum Leaching System Spread (MLSS) process to site leaching systems on any individual parcel. If more than more one system were proposed on a lot, the stacking of these systems would be considered if located less than 50 feet of each other up or down gradient. It is the applicant's opinion that hydraulic considerations are not serious due to the well drained soils and that MLSS is not applicable because no restrictive layers were observed within 60" of the

ground surface. Most of the percolation tests performed on these parcels were at relatively shallow depths and in order to assume the non-existence of a restrictive layer down to 60 inches, at least several additional deep percolation tests must be performed to identify a seepage rate of 30 minutes per inch or faster. Hydraulic analysis may still be requested if the affects of stacking the leaching systems is identified in final plans.

- Water Supply Protection - Each of the eighteen buildings will be served by separate sewage disposal systems and wells. Sewage disposal systems and wells are scattered throughout the three parcels. Highly permeable soils underlain by bedrock with moderate slopes and septic systems uphill from proposed well sites will create a serious potential for partially treated sewage to enter one or more of these wells. This is particularly true with respect to startup of the project prior to developing a restrictive organic mat at the interface where leachate comes in contact with the soil. Obviously, there would be significant advantages to locating wells upgradient from sewage disposal systems to minimize any potential adverse impacts. Obviously, proper well construction would be critical for all wells located down gradient from potential sources of pollution and the benefits of providing a public water supply would most likely be discussed by engineers and planners in our water supplies section who will be reviewing the proposal pursuant to CGS 25-32.

To summarize, the high density of development on each of the three parcels have generated concern for effective long term onsite sewage disposal and protection of wells. All onsite systems degrade water quality immediately adjacent to the leaching systems and with larger systems in greater discharges, the impact of the pollution plume may extend significantly beyond the direct areas of application. The State Department of Public Health's responsibility, when reviewing final plans for project development, will be to make the determination which will assure long term protection of ground and surface waters as well as protection of public water supplies proposed on each of the three parcels.

ABOUT THE TEAM

The King's Mark Environmental Review Team (ERT) is a group of environmental professionals drawn together from a variety of federal, state and regional agencies. Specialists on the Team include geologists, biologists, soil scientists, foresters, climatologists and landscape architects, recreational specialists, engineers and planners. The ERT operates with state funding under the aegis of the King's Mark Resource Conservation and Development (RC&D) Area - an 83 town area serving western Connecticut.

As a public service activity, the Team is available to serve towns within the King's Mark RC&D Area - **free of charge.**

Purpose of the Environmental Review Team

The Environmental Review Team is available to assist towns in the review of sites proposed for major land use activities or natural resource inventories for critical areas. For example, the ERT has been involved in the review of a wide range of significant land use activities including subdivisions, sanitary landfills, commercial and industrial developments and recreation/open space projects.

Reviews are conducted in the interest of providing information and analysis that will assist towns and developers in environmentally sound decision making. This is done through identifying the natural resource base of the site and highlighting opportunities and limitations for the proposed land use.

Requesting an Environmental Review

Environmental reviews may be requested by the chief elected official of a municipality or the chairman of an administrative agency such as planning and zoning, conservation or inland wetlands. Environmental Review Request Forms are available at your local Soil and Water Conservation District and through the King's Mark ERT Coordinator. This request form must include a summary of the proposed project, a location map of the project site, written permission from the landowner/developer allowing the Team to enter the property for the purposes of a review and a statement identifying the specific areas of concern the Team members should investigate. When this request is reviewed by the local Soil and Water Conservation District and approved by the King's Mark RC&D Executive Council, the Team will undertake the review. At present, the ERT can undertake approximately two reviews per month depending on scheduling and Team member availability.

For additional information regarding the Environmental Review Team, please contact the King's Mark ERT Coordinator, Connecticut Environmental Review Team, P.O. Box 70, Haddam, CT 06438. The telephone number is 860-345-3977.